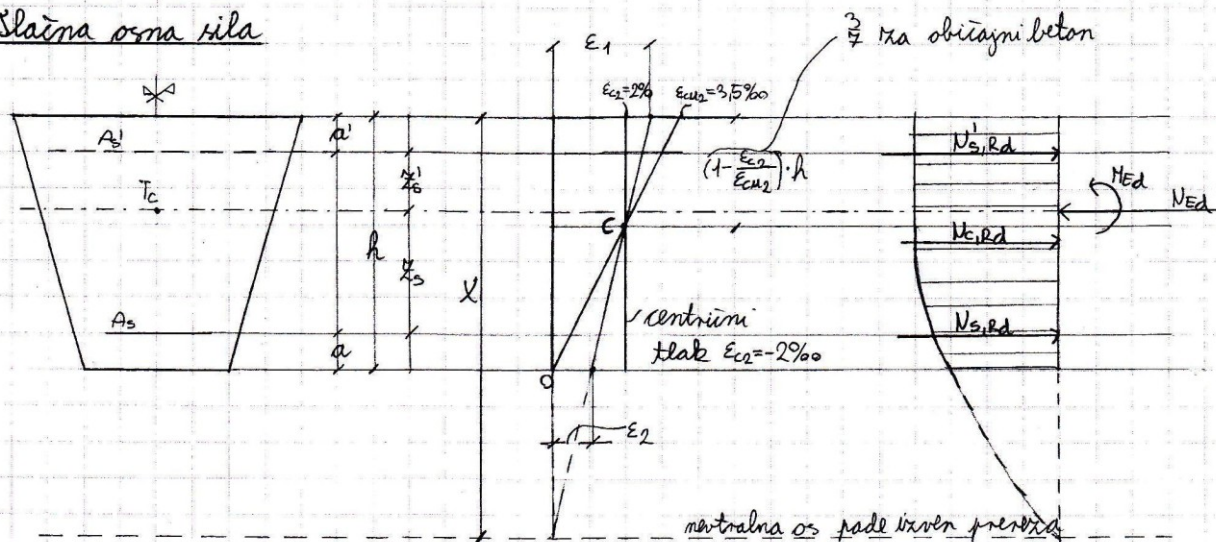


1.2. Osnovna upogibna obremenitev - mala ekscentričnost (po celotnem prerezu napetosti in deformaciji enakega predznaka)

- tlačna ali tlačna osna sila; po celotnem prerezu deformaciji enakega predznaka
- sbrani obremenitvi (centrični tlak, centrični nateg)

Tlačna osna sila



$$\delta = \frac{a}{h}$$

A'_s ... bolj tlačna armatura

A_s ... manj tlačna armatura

$e = \frac{M_{Ed}}{N_{Ed}} \Rightarrow \bar{e} < \text{jedra preseka } j \text{ (za } \square = h/6) \Rightarrow \text{potem mala ekscentričnost}$

3 predpisanim $\epsilon_2 \rightarrow$ določen potek deformacij $\epsilon_1 = \epsilon_{cu2} - \frac{3}{4} \epsilon_2$

$$x = k_x \cdot h \quad k_x = \frac{\epsilon_1}{\epsilon_1 - \epsilon_2}$$

$$\epsilon_s = \frac{k_x - 1 + \delta}{k_x - 3/4} \cdot \epsilon_{c2} \leq 2\text{‰}$$

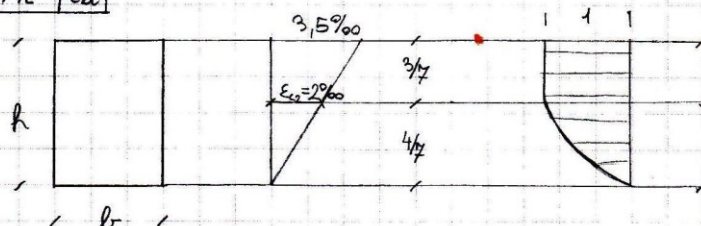
$$\sigma_s = \sigma_s(\epsilon_s) \quad \sigma'_s = \sigma'_s(\epsilon'_s)$$

$$|\epsilon_s| < \epsilon_{yd} \rightarrow \sigma_s = E_s \cdot \epsilon_s$$

$$|\epsilon_s| > \epsilon_{yd} \rightarrow \sigma_s = f_{yd}$$

$$N_{c,Ed} = m_{cd} \cdot A_c \cdot f_{cd}$$

↳ majhni primer



$$m_{cd} = \underbrace{\left(\frac{3}{4} \cdot 1 + \frac{2}{3} \cdot 1 \cdot \frac{4}{4} \right)}_{\text{ploščina}} = 0.8095 \quad \text{glej stari priručnik (tabela 7 str. 211; } m_{cu} \text{)}$$

Primer: pravolastni prerez

Znane so dimenzije preseka in razmerje armature. Uporaba interakcijskega diagrama.

$$M_{Ed} = -6825 \text{ kNm (tlak)} \quad M_{Ed} = 279 \text{ kNm} \quad k = \frac{A_s}{A_c} = 1,0$$

$$C40/50 \rightarrow f_{cd} = \frac{4}{1,5} = 2,67 \text{ kN/cm}^2 \quad S400 \rightarrow f_{yd} = \frac{40}{1,15} = 34,8 \text{ kN/cm}^2 \text{ (stari prirönik)}$$

$$b = 45 \text{ cm} \quad h = 65 \text{ cm} \quad a = a' = 4,5 \text{ cm}$$

$$A_c = b \cdot h = 45 \cdot 65 = 2925 \text{ cm}^2$$

$$j = \frac{a}{h} = \frac{4,5}{65} = 0,07$$

$$m_{Ed} = \frac{N_{Ed}}{A_c \cdot f_{cd}} = \frac{-6825}{2925 \cdot 2,67} = -0,874$$

$$m_{Ed} = \frac{M_{Ed}}{A_c \cdot h \cdot f_{cd}} = \frac{279 \cdot 100}{2925 \cdot 65 \cdot 2,67} = 0,055$$

V starem priröniku odčitam za S400, $k=1$, $j=0,07$

$$\mu_0 \approx 0,05$$

$$\mu = \frac{\mu_0}{1+k} \cdot \frac{f_{cd}}{f_{yd}} \quad \mu = \frac{0,05}{1+1,0} \cdot \frac{2,67}{34,8} = 0,0019$$

$$A_s = \mu \cdot A_c = 0,0019 \cdot 2925 = 5,6 \text{ cm}^2 \quad (2 \phi 19; A_{s,dej} = 5,67 \text{ cm}^2)$$

$$A_s' = k \cdot A_s = 1,0 \cdot 5,6 = 5,6 \text{ cm}^2 \quad (2 \phi 19; A_{s',dej} = 5,67 \text{ cm}^2)$$

Drugi primer

Identino k prejšnjem primeru razen S500 $\rightarrow f_{yd} = 43,48 \text{ kN/cm}^2$

$$A_c = 2925 \text{ cm}^2 \quad j = 0,07 \quad m_{Ed} = -0,874 \quad m_{Ed} = 0,055$$

V novem priröniku odčitam za S500, $k=1$, $j=0,07 \rightarrow j=0,10$ ali $j=0,05$ ~ kar da več armature
 $j=0,10 \Rightarrow \mu_0 = 0,1 \Rightarrow \mu = \frac{\mu_0}{1+k} \cdot \frac{f_{cd}}{f_{yd}} = \frac{0,1}{1+1,0} \cdot \frac{2,67}{43,48} = 0,0031$ (da manj)

$$A_s = \mu \cdot A_c = 0,0031 \cdot 2925 = 9,0 \text{ cm}^2 \quad (4 \phi 17; A_{s,dej} = 9,08 \text{ cm}^2)$$

$$A_s' = k \cdot A_s = 1,0 \cdot 9,0 = 9,0 \text{ cm}^2 \quad (4 \phi 17; A_{s',dej} = 9,08 \text{ cm}^2)$$

Minimalna armatura

- stebri ($b \leq 4h$)

$$A_{s,min} = \max \left(\frac{0,1 \cdot N_{Ed}}{f_{yd}}; 0,002 A_c \right)$$

- nosilni plošöe

$$A_{s,min} = \max \left(\frac{0,26 f_{ctm}}{f_{yk}} b \cdot d; 0,0013 b \cdot d \right)$$

karaktistiöna S500 $\rightarrow f_{yk} = 50 \text{ kN/cm}^2$

b_t ... mednja širina matzne cone

f_{ctm} ... matzna trdnost betona

Maksimalna armatura

• na vse

$A_{s,max} = 0,04 A_c$; če previdimo pogoj $\rightarrow$ povečanje dimenzij, boljši material,...

Centrični tlak

• dvojno simetrični presezi

Bravolotni presek

C 30/37 $\rightarrow f_{cd} = 2,0 \text{ kN/cm}^2$ $E_c = E_{cz} = -2\%$ mejna def.

S500 $\rightarrow \sigma_s = \sigma_s(\epsilon_s) = E_s \cdot \epsilon_s = 20000 \cdot 0,002 = \underline{40 \text{ kN/cm}^2}$

$$\sigma_c = f_{cd} = 2,0 \text{ kN/cm}^2$$

$$N_{Ed} = 5000 \text{ kN}$$

$$M_{Ed} = 0 \text{ kNm}$$

$$b/h = 45/50 \text{ cm}$$

$$N_{Ed} = A_c \cdot f_{cd} + \Sigma A_s \cdot \sigma_s$$

$$\Sigma A_s = (N_{Ed} - A_c \cdot f_{cd}) / \sigma_s = (5000 - 45 \cdot 30 \cdot 2,0) / 40 = \underline{12,5 \text{ cm}^2} \rightarrow \text{če pride presek armature}$$

negativen pomeni, da svo obtežbo prevzame beton, v preseku pa damo $A_{s,min}$.

Preverim $\Sigma A_{s,min}$ in $\Sigma A_{s,max}$

$$\Sigma A_{s,min} = \max\left(\frac{0,1 \cdot N_{Ed}}{f_{yd}}; 0,002 A_c\right) = \max(11,5; 4,5) = \underline{11,5 \text{ cm}^2}$$

$$\Sigma A_{s,max} = 0,04 A_c = 0,04 \cdot 45 \cdot 50 = \underline{90 \text{ cm}^2}$$

• Izračunan presek armature ΣA_s ustreza tistemu pogojema

$$\Sigma A_s = 12,5 \text{ cm}^2 \quad (4 \phi 20; A_{s,dej} = 12,57 \text{ cm}^2)$$

Število armaturnih palic v preseku

- v kvadratnem preseku vsaj ena palica v vsakem vogalu
- v obročnem preseku vsaj 4 armaturne palice

• enojno simetričen presek

T-presek

Diagram of a T-joint weld configuration. The top plate has a width of 70 mm and a thickness of 5 mm. The vertical plate has a height of 35 mm and a thickness of 5 mm. The welds are labeled A2, A3, and Tc=Ts. The total length of the joint is 35 mm.

$$N_{Ed} = -12000 \text{ kV (tlaq)} \quad M_{Ed} = 0$$

$$C40/45 \leadsto f_{cd} = 2,67 \text{ kN/cm}^2$$

$$R_g = R_g - \bar{S} = 40,8 - \bar{S} = \underline{35,8 \text{ km}}$$

$$N_{Ed} = A_c \cdot f_{cd} + \sum A_s \cdot \sigma_s \leadsto \sum A_s = (N_{Ed} - A_c \cdot f_{cd}) / \sigma_s = (12000 - 3673 \cdot 2,67) / 40 = \underline{54,7 \text{ cm}^2}$$

1. pagoj $A_S \cdot K_S = A'_S \cdot K'_S$

2. pagej $\Sigma A_s = A_s + A'_s$

$$A_S = \sum A_S - A_S' \rightarrow Z_S (\sum A_S - A_S') = A_S' \cdot Z_S'$$

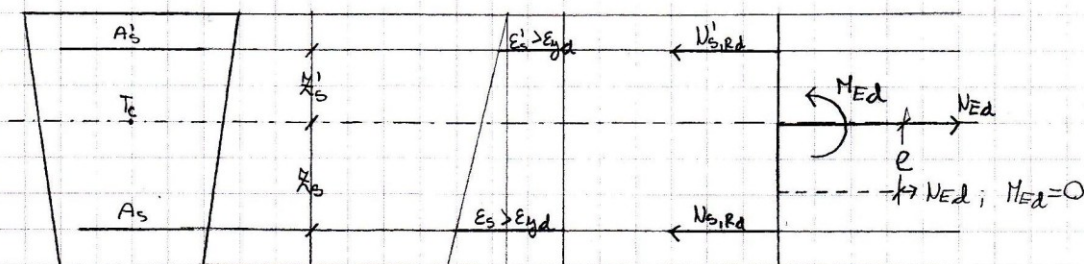
$$A'_S (Z_S + Z'_S) = \sum A'_S \cdot Z_S$$

$$A'_s = \frac{\sum A_s R_s}{R'_6 + R_s}$$

$$A_s' = \frac{54,7 \cdot 35,8}{\underbrace{24,2 + 35,8}_b} = \underline{\underline{32,6 \text{ cm}^2}}$$

$$A_s = \sum A_s - A'_s = 54,7 - 32,6 = \underline{22,1 \text{ m}^2}$$

$\epsilon > 0 \leadsto$ beton pri MSN „ne razdeljuje“



$$e = \frac{MEd}{NEd}$$

↳ odmik armature od težišća

1.) Momentni ravnotežni pogoj na N_{srd} :

$$N_{Ed} \cdot (K'_s + e) - N_{S,Rd} \cdot (K'_s + K_s) = 0 \Rightarrow N_{S,Rd} = N_{Ed} \cdot \frac{K'_s + e}{K'_s + K_s}$$

$$A_s = \frac{N_{s, Rd}}{f_{yd}}$$

$$A_s = \frac{N_{Ed}}{f_{yd}} \cdot \frac{x'_s + e}{x'_s + x_s}$$

2.) Momentni ravnotežni pogoj na $N_{s,rd}$:

$$N'_{s,rd} \cdot (\kappa_s + \kappa'_s) - N_{Ed} \cdot (\kappa_s - e) = 0 \Rightarrow N'_{s,rd} = N_{Ed} \cdot \frac{\kappa_s - e}{\kappa_s + \kappa'_s}$$

$$A_s = \frac{N'_{s,rd}}{f_{yd}}$$

$$A_s = \frac{N_{Ed}}{f_{yd}} \cdot \frac{\kappa_s - e}{\kappa_s + \kappa'_s}$$

Se enačbe veljajo, če je $e \leq \kappa_s$ oziroma $e \leq \kappa'_s$

Centrični natež $e=0$

• nesimetrični presek

$$A_s = \frac{N_{Ed}}{f_{yd}} \cdot \frac{\kappa'_s}{\kappa_s + \kappa'_s}$$

V zgornji plošni enačbi za A_s in A'_s

vtarimo $e=0$

$$A'_s = \frac{N_{Ed}}{f_{yd}} \cdot \frac{\kappa_s}{\kappa_s + \kappa'_s}$$

• simetrični presek

$$\kappa_s = \kappa'_s$$

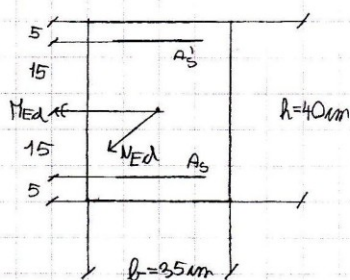
$$A_s = A'_s = \frac{N_{Ed}}{2f_{yd}}$$

Postavitve armature

• centrični natež: Enakomerno po preseku (po obodu) $T_s \approx T_c$

• ekscentrični natež: Sežisek vse armature se ujema s prijemališčem sile
 $N_{Ed}(e)$

Primer: ekscentrični natež - simetrični presek



$$S500 \leadsto f_{yd} = 43,48 \text{ kN/mm}^2$$

$$C30/37 \leadsto \sigma_{ct,Ed} = 2,0 \text{ kN/mm}^2$$

Obremenitev: Stalna $N_g = 500 \text{ kN}$, $M_g = 15 \text{ kNm}$

Spremenljiva $N_q = 200 \text{ kN}$, $M_q = 10 \text{ kNm}$

$$N_{Ed} = 1,35 \cdot 500 + 1,5 \cdot 200 = 975 \text{ kN}$$

$$M_{Ed} = 1,35 \cdot 15 + 1,5 \cdot 10 = 35,25 \text{ kNm}$$

$$e = \frac{M_{Ed}}{N_{Ed}} = 35,25 / 975 = 0,036 \text{ m}; \kappa_s = \kappa'_s = 0,15 \text{ m} > e \Rightarrow \text{mala ekscentričnost}$$

Če bi bil $e > \kappa_s = \kappa'_s$ bi računali po enačbah za veliko ekscentričnost (kd...)

$$A_s = \frac{N_{Ed}}{f_{yd}} \cdot \frac{\kappa'_s + e}{\kappa_s + \kappa'_s} = \frac{975}{43,48} \cdot \frac{15 + 0,036}{15 + 15} = 13,9 \text{ mm}^2$$

$$A'_s = \frac{N_{Ed}}{f_{yd}} \cdot \frac{\kappa_s - e}{\kappa_s + \kappa'_s} = \frac{975}{43,48} \cdot \frac{15 - 0,036}{15 + 15} = 8,52 \text{ mm}^2$$

Dimenzije betonskega preseka določimo po MSU (mejno stanje uporabnosti)
oziroma je potrebno preveriti, ena izmed MSU je mejno stanje razpok.